

TITLE 24, PART 2
CHAPTER 16

1645B.3.1.4 Any element not meeting the requirements of Sections 1645B.3.1.1, 1645B.3.1.2 or 1645B.3.1.3 shall be classified as non-ductile, with corresponding ϕ value equal to or greater than that given in Table 16B-R-1, except where Section 1645B.2 allows use of another value. The Section 1645B.2.2 analysis shall consider at a minimum:

1. Reinforcing bar lap splice length, cover, and ties.
2. Pile-to-footing connection resistance to tension due to overturning moment. (Section 1644B.9.3, Part 2, Title 24, California Code of Regulations, ~~1995 Edition~~).
3. Footing flexural and shear capacity.
4. Column ties for both shear resistance and concrete confinement.
5. Positive Moment tension bar pullout or slab flexural failure. (Section 1646B.1.3.2)
6. Negative moment hook pullout.
7. Stirrups for both shear resistance and concrete confinement.
8. Non-continuous longitudinal steel leaving sections with weakness in flexural and shear resistance. (1994 UBC Section 1921.8.4.1)
9. Joint shear reinforcing and confinement.
10. Weak column-strong beam condition. (Section 1921B.4.2.2, Part 2, Title 24, California Code of Regulations, ~~1995 Edition~~ and 1645B.3.2 Exception 2).
11. Slab punching shear.
12. Short or captive column.
13. The shear capacity of columns.

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1645B.4 Masonry

1645B.4.1 Ductile or Code Complying: Any element in essential conformance with the seismic requirements of Chapter 21B, Sections 2106B.1.12.4 and 2108B.2.3.8, Part 2, Title 24, California Code of Regulations, ~~1995 Edition~~, may be classified as ductile and the α value taken as 1.0.

Exception: Any shear wall pier and spandrel element having height or clear span to depth ratios greater than two shall either comply with Section 2108B.2.6 (Wall Frames), Part 2, Title 24, California Code of Regulations, ~~1995 Edition~~, to be classified as ductile; otherwise, it shall be classified as a limited-ductile element with $\alpha = 2.5$ or greater.